

# **2.019 Design of Ocean Systems**

## **Lecture 5**

### **Seakeeping (I)**

**February 18, 2011**

# Six-Degree-of-Freedom Motion of a Floating Body in Waves

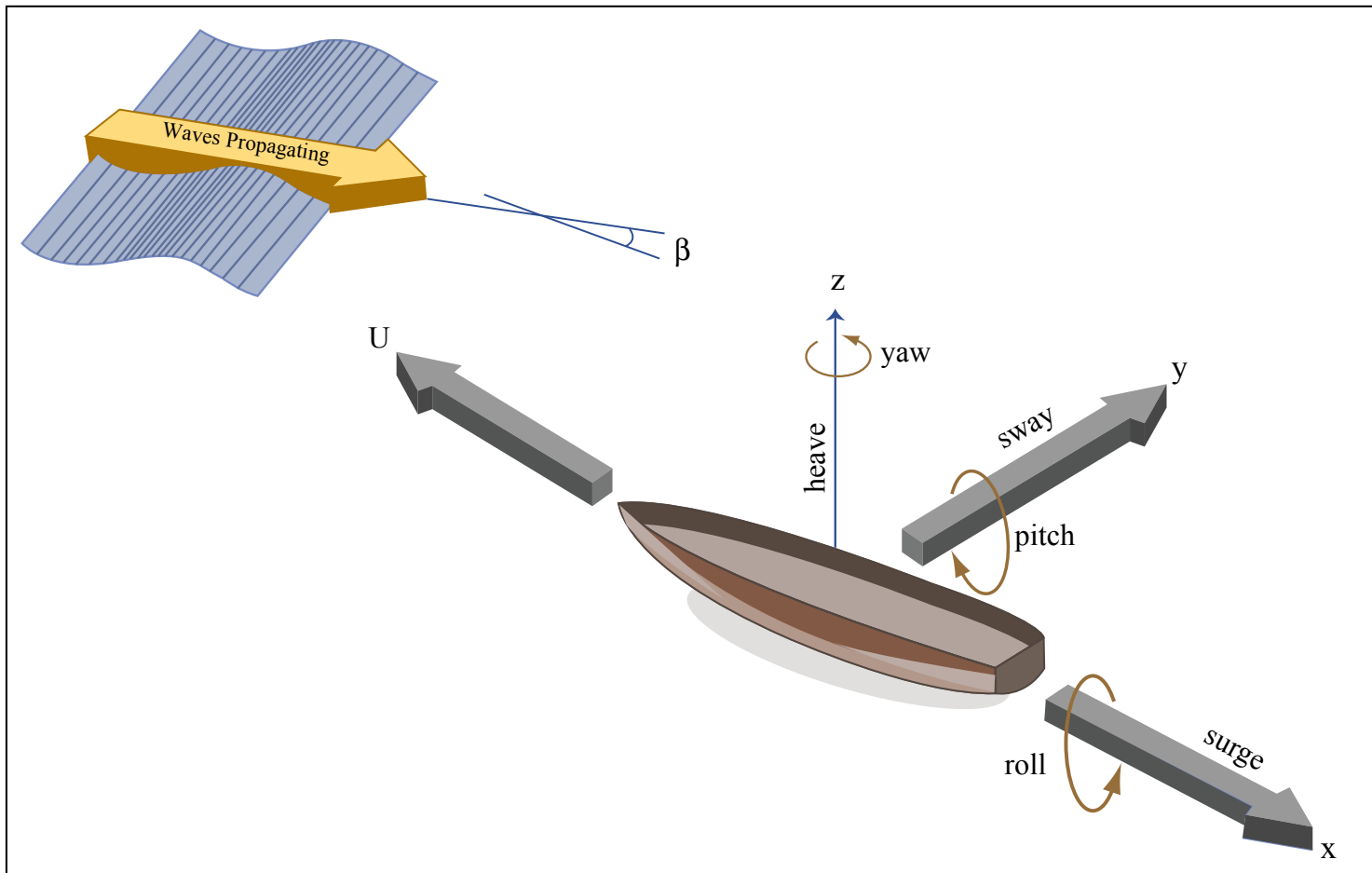
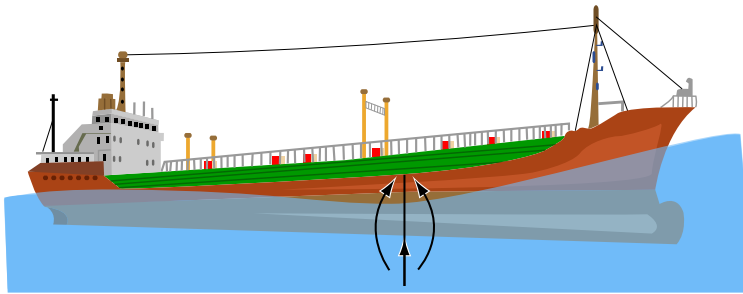


Image by MIT OpenCourseWare.

Translation in x:	surge	$\zeta_1(t)$
Translation in y:	sway	$\zeta_2(t)$ ;
Translation in z:	heave	$\zeta_3(t)$ ;
Rotation with x:	roll	$\zeta_4(t)$ ;
Rotation with y:	pitch	$\zeta_5(t)$ ;
Rotation with z:	yaw	$\zeta_6(t)$ ;

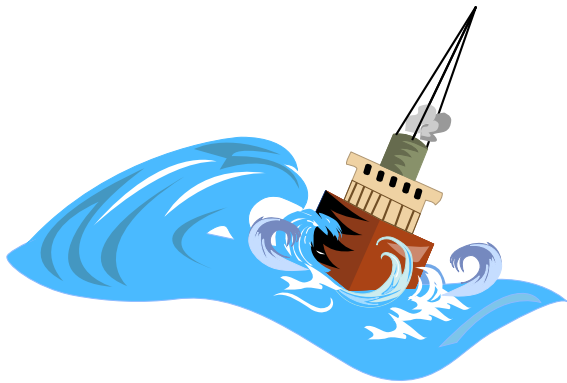
# Examples of Seakeeping and Wave Load Problems for Ships and Offshore Structure



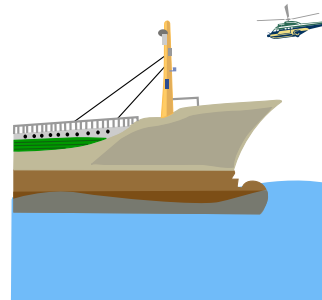
Wave bending moments and shear forces



Accelerations



Effect of breaking waves



Local motions



Water on deck



Liquid sloshing in Tanks

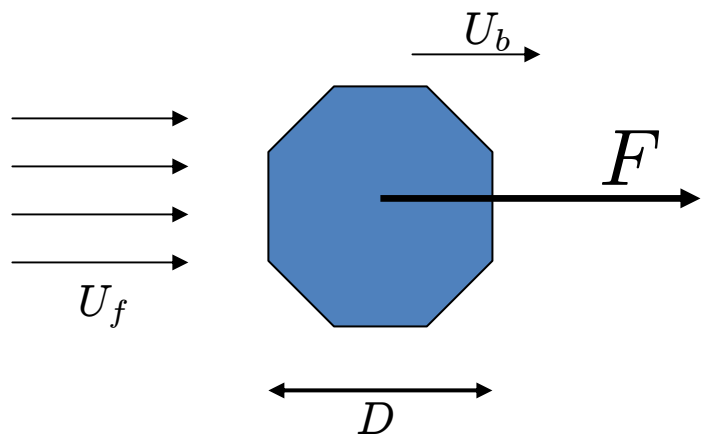


Slamming

# Concerns of Seakeeping in FPSO Design

- Increasing maximum loads (due to dynamic pressure)
- Affecting operation
  - Production by Risers
  - Gas-oil, oil/water separation
  - Normally, heave amplitude  $< 4\text{m}$ , pitch amplitude  $< 5$  degrees, roll amplitude  $< 10$  degrees, excursion  $< (5\sim 8)\%$  water depth
- Vibration of superstructures
- Fatigue life of hull structures, risers, etc.
- Survival in extreme seas
- Local extreme structure damage (bottom slamming, breaking wave impact, green water on deck etc.)
- Human safety

# Hydrodynamic Forces on a Body in Unbounded Fluid



$$(1) U_f = 0, U_b(t) \neq 0$$

$$F(t) = -m_a \frac{dU_b(t)}{dt}$$

$m_a$  : Added mass  
 Depending on body geometry, motion direction, fluid density

$$(2) U_f(t) \neq 0, U_b = 0$$

**Morrison's formula:**

$$F(t) = \rho \nabla \frac{dU_f(t)}{dt} + m_a \frac{dU_f(t)}{dt}$$

Froude-Krylov force

Added mass effect

$\rho$ : Fluid density  
 $\nabla$ : Body volume

$$(3) U_f(t) \neq 0, U_b(t) \neq 0$$

$$F(t) = \rho \nabla \frac{dU_f(t)}{dt} + m_a \left\{ \frac{dU_f(t)}{dt} - \frac{dU_b(t)}{dt} \right\}$$

# Potential Flow

- In typical marine engineering applications such as ships, offshore platforms,

$$R_e = \frac{UL}{\nu} = 10^6 \sim 10^7$$

Thus, viscous effect can be neglected in general.

- Flow can be considered as a irrotational flow (i.e. vorticity  $\nabla \times \vec{v} = 0$ ) except under some special conditions where flow separation occurs.
- Fluid motion in the ocean is normally assumed as a potential flow:

$$\text{Velocity: } \vec{v}(x, y, z, t) = \nabla \phi(x, y, z, t)$$

$$\text{Continuity equation: } \nabla^2 \phi = 0$$

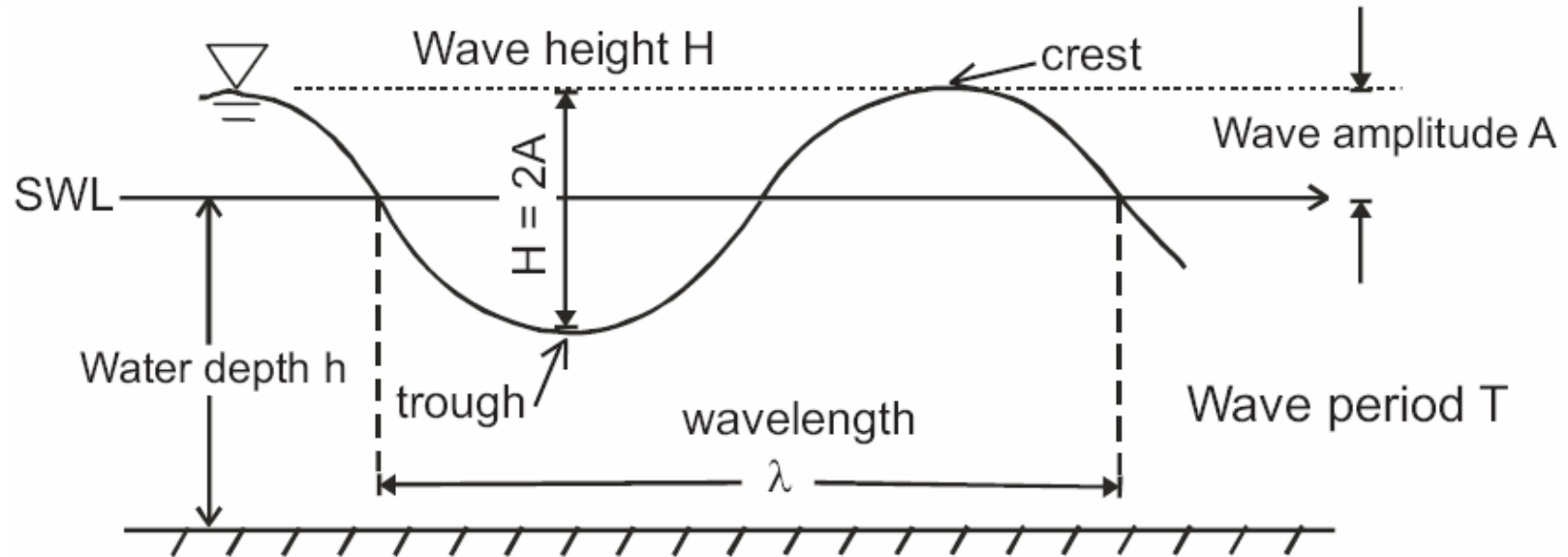
$$\text{Momentum equation: } \frac{p(x, y, z, t)}{\rho} = -\frac{\partial \phi}{\partial t} - \frac{1}{2} |\nabla \phi|^2 - gz$$

- The key is to solve the Laplace equation with certain boundary conditions for the velocity potential  $\phi(x, y, z, t)$

# Linearized (Airy) Wave Theory

Assume small wave amplitude compared to wavelength, i.e., small free surface slope

$$\frac{A}{\lambda} \ll 1$$



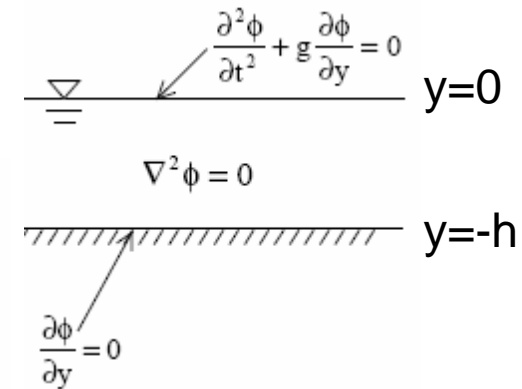
Consequently

$$\frac{\phi}{\lambda^2/T}, \frac{\eta}{\lambda} \ll 1$$

We keep only linear terms in  $\phi$ ,  $\eta$ .

For example:  $(\ )|_{y=\eta} = \underbrace{(\ )|_{y=0}}_{\text{keep}} + \eta \underbrace{\frac{\partial}{\partial y} (\ )|_{y=0}}_{\text{discard}} + \dots$  Taylor series

- Boundary-Value Problem (BVP) for linearized (Airy) wave:



	Finite depth $h = \text{const}$	Infinite depth
GE:	$\nabla^2 \phi = 0, \quad -h < y < 0$	$\nabla^2 \phi = 0, \quad y < 0$
BKBC:	$\frac{\partial \phi}{\partial y} = 0, \quad y = -h$	$\nabla \phi \rightarrow 0, \quad y \rightarrow -\infty$
FSKBC:	$\frac{\partial \phi}{\partial y} = \frac{\partial \eta}{\partial t}, \quad y = 0$	} $\rightarrow \frac{\partial^2 \phi}{\partial t^2} + g \frac{\partial \phi}{\partial y} = 0$
FSDBK:	$\frac{\partial \phi}{\partial t} + g\eta = 0, \quad y = 0$	

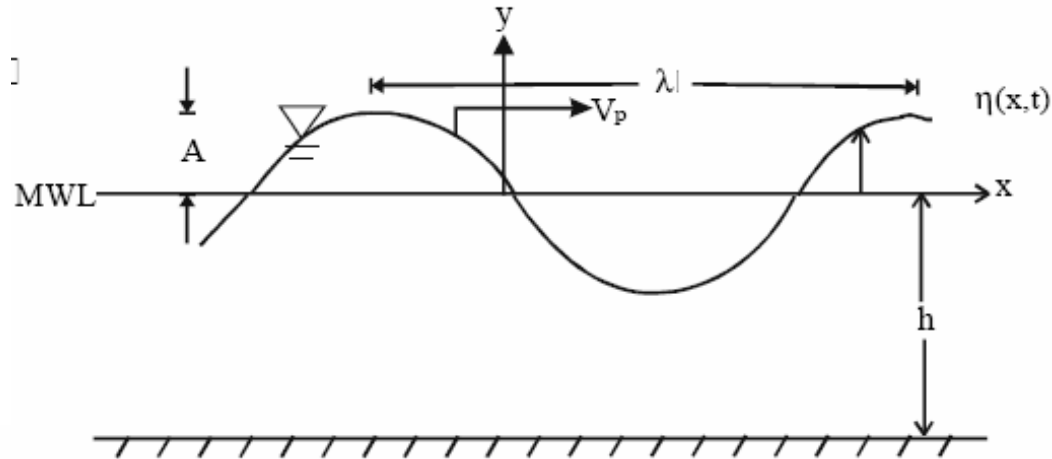
- Given velocity potential  $\phi$ , find free-surface elevation  $\eta$  and pressure  $p$ :

$$\eta(x, t) = -\frac{1}{g} \left. \frac{\partial \phi}{\partial t} \right|_{y=0}$$

$$p - p_a = \underbrace{-\rho \frac{\partial \phi}{\partial t}}_{\text{dynamic}} - \underbrace{\rho g y}_{\text{hydrostatic}}$$



# Solution of 2D Periodic Progressive (Airy) Waves



Potential: 
$$\phi = \frac{gA}{\omega} \sin(kx - \omega t) \frac{\cosh k(y + h)}{\cosh kh}$$

Free-surface elevation: 
$$\eta = A \cos(kx - \omega t)$$

$A = H/2$ : wave amplitude;  $k = 2\pi/\lambda$ : wavenumber;  $\omega = 2\pi/T$ : frequency

Dispersion relation: 
$$\omega^2 = gk \tanh kh$$

Phase velocity: 
$$V_p \equiv \frac{\lambda}{T} = \frac{\omega}{k} = \sqrt{\frac{g}{k} \tanh kh}$$

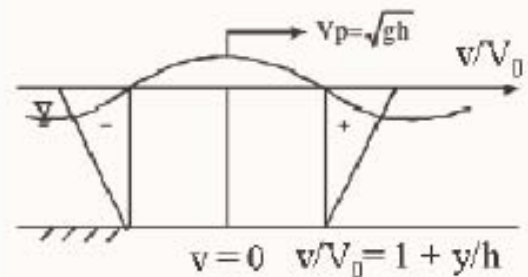
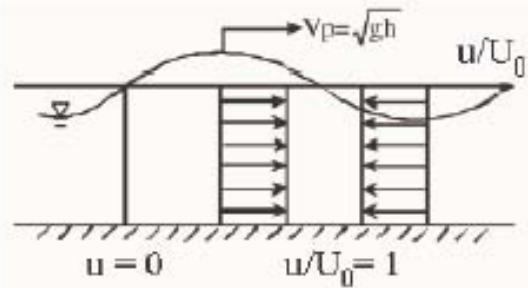
# Characteristics of a Linear Plane Progressive Wave

- Velocity Field:

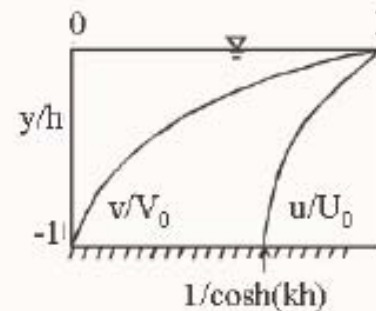
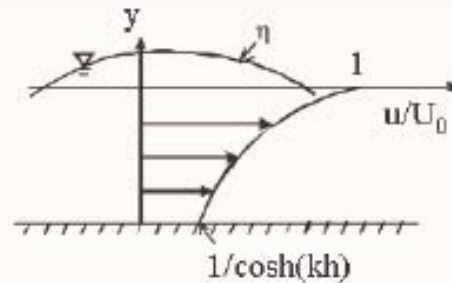
Velocity on free surface $\vec{v}(x, y = 0, t)$	
$u(x, 0, t) \equiv U_o = A\omega \frac{1}{\tanh kh} \cos(kx - \omega t)$	$v(x, 0, t) \equiv V_o = A\omega \sin(kx - \omega t) = \frac{\partial \eta}{\partial t}$
Velocity field $\vec{v}(x, y, t)$	
$u = \frac{\partial \phi}{\partial x} = \frac{Agk \cosh k(y+h)}{\omega \cosh kh} \cos(kx - \omega t)$ $= \underbrace{A\omega}_U \frac{\cosh k(y+h)}{\sinh kh} \cos(kx - \omega t) \Rightarrow$	$v = \frac{\partial \phi}{\partial y} = \frac{Agk \sinh k(y+h)}{\omega \cosh kh} \sin(kx - \omega t)$ $= \underbrace{A\omega}_U \frac{\sinh k(y+h)}{\sinh kh} \sin(kx - \omega t) \Rightarrow$
$\frac{u}{U_o} = \frac{\cosh k(y+h)}{\cosh kh} \begin{cases} \sim e^{ky} & \text{deep water} \\ \sim 1 & \text{shallow water} \end{cases}$	$\frac{v}{V_o} = \frac{\sinh k(y+h)}{\sinh kh} \begin{cases} \sim e^{ky} & \text{deep water} \\ \sim 1 + \frac{y}{h} & \text{shallow water} \end{cases}$
<ul style="list-style-type: none"> <li>• <math>u</math> is in phase with <math>\eta</math></li> </ul>	<ul style="list-style-type: none"> <li>• <math>v</math> is out of phase with <math>\eta</math></li> </ul>

## Velocity field $\vec{v}(x, y)$

Shallow water



Intermediate water

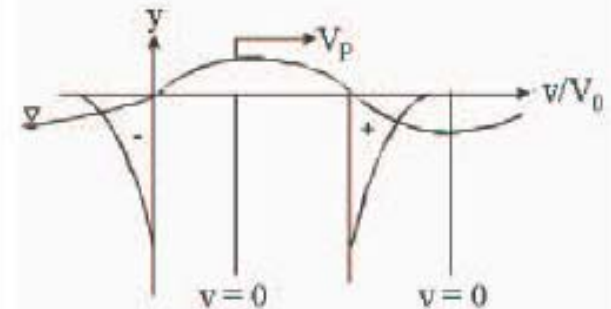
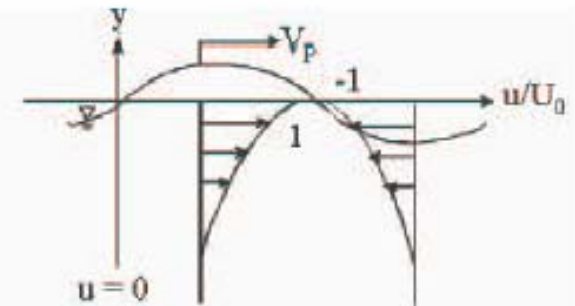


Shallow water / Long waves:  $kh \ll 1$

$$u = \frac{A\omega}{kh} \cos(kx - \omega t) = \eta \sqrt{\frac{g}{h}}$$

$$v = A\omega \left(1 + \frac{y}{h}\right) \sin(kx - \omega t)$$

Deep water



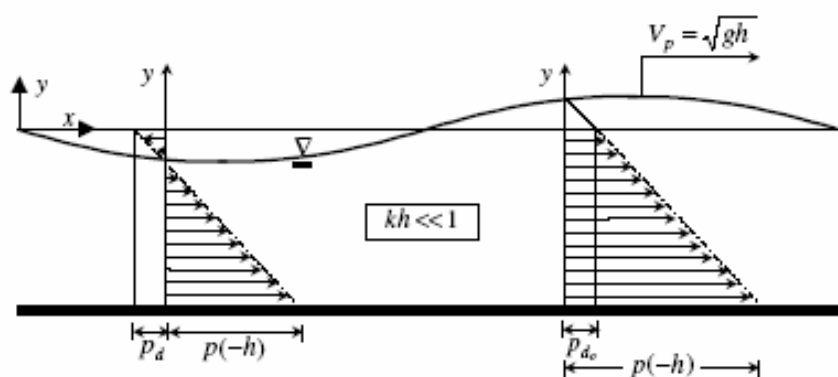
Rule of thumb  $\frac{u}{u_s} = \frac{v}{v_s} = 4\%$  at  $y = -\frac{\lambda}{2}$   
 $(\cosh kh - 1, \sinh kh - kh)$

• Pressure Field:

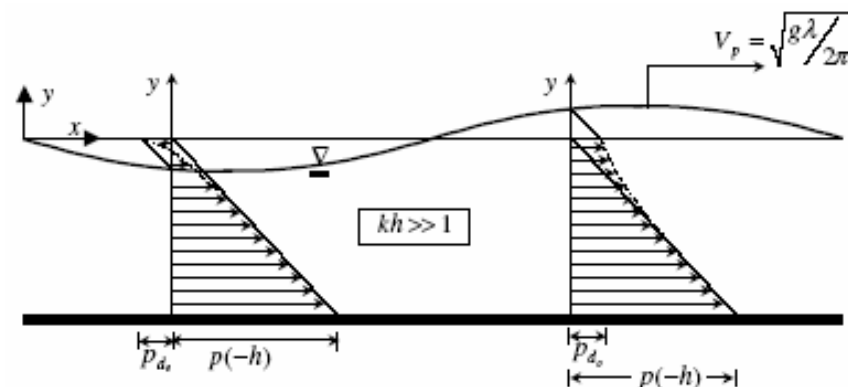
$$\text{Total pressure } p = p_d - \rho g y.$$

$$\text{Dynamic pressure } p_d = -\rho \frac{\partial \phi}{\partial t}.$$

Pressure field		
Shallow water	Intermediate water	Deep water
$p_d = \rho g \eta$	$p_d = \rho g A \frac{\cosh k(y+h)}{\cosh kh} \cos(kx - \omega t)$ $= \rho g \frac{\cosh k(y+h)}{\cosh kh} \eta$	$p_d = \rho g e^{ky} \eta$
$\frac{p_d}{p_{d_0}}$ same picture as $\frac{u}{U_0}$		
$\frac{p_d(-h)}{p_{d_0}} = 1$ (no decay)	$\frac{p_d(-h)}{p_{d_0}} = \frac{1}{\cosh kh}$	$\frac{p_d(-h)}{p_{d_0}} = e^{-ky}$
$p = \underbrace{\rho g(\eta - y)}_{\text{"hydrostatic" approximation}}$		$p = \rho g (\eta e^{ky} - y)$



Pressure field in shallow water



Pressure field in deep water

# Wave Energy

For a single plane progressive wave:

Energy per unit surface area of wave	
• Potential energy PE	• Kinetic energy KE
PE without wave = $\int_{-h}^0 \rho g y dy = -\frac{1}{2} \rho g h^2$	$KE_{wave} = \int_{-h}^{\eta} dy \frac{1}{2} \rho (u^2 + v^2)$
PE with wave $\int_{-h}^{\eta} \rho g y dy = \frac{1}{2} \rho g (\eta^2 - h^2)$	Deep water = ... = $\underbrace{\frac{1}{4} \rho g A^2}_{KE \text{ const in } x, t}$ to leading order
$PE_{wave} = \frac{1}{2} \rho g \eta^2 = \frac{1}{2} \rho g A^2 \cos^2(kx - \omega t)$	Finite depth = ...
Average energy over one period or one wavelength	
$\overline{PE}_{wave} = \frac{1}{4} \rho g A^2$	$\overline{KE}_{wave} = \frac{1}{4} \rho g A^2$ at any $h$

- Total wave energy in **deep** water:

$$E = PE + KE = \frac{1}{2} \rho g A^2 \left[ \cos^2(kx - \omega t) + \frac{1}{2} \right]$$

- Average wave energy  $E$  (over 1 period or 1 wavelength) for **any** water depth:

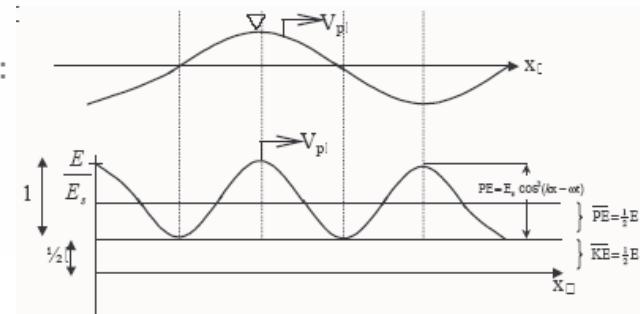
$$\overline{E} = \frac{1}{2} \rho g A^2 \left[ \underbrace{\frac{1}{2}}_{\overline{PE}} + \underbrace{\frac{1}{2}}_{\overline{KE}} \right] = \frac{1}{2} \rho g A^2 = E_s,$$

$E_s \equiv$  Specific Energy: total average wave energy per unit surface area.

- Linear waves:  $\overline{PE} = \overline{KE} = \frac{1}{2} E_s$   
(equipartition).

- Nonlinear waves:  $\overline{KE} > \overline{PE}$ .

- Wave energy propagation speed: group velocity:  $V_g = \frac{d\omega}{dk}$



Recall:  $\cos^2 x = \frac{1}{2} + \frac{1}{2} \cos 2x$

## Example: Wave Loads on Vertical Wall

A vertical wall is located at  $x=0$  in a water of depth  $h$ :

$$\eta(x, t) = A \cos(kx - \omega t) + A \cos(kx + \omega t)$$

$$\phi(x, y, t) = \frac{gA}{\omega} \frac{\cosh k(y+h)}{\cosh kh} [\sin(kx - \omega t) - \sin(kx + \omega t)]$$

$$p(x, y, t) = -\rho \frac{\partial \phi}{\partial t} - \rho g y = \rho g A \frac{\cosh k(y+h)}{\cosh kh} [\cos(kx - \omega t) + \cos(kx + \omega t)] - \rho g y$$

$$p(x = 0, y, t) = 2\rho g A \cos \omega t \frac{\cosh k(y+h)}{\cosh kh} - \rho g y$$

$$F_x = \int_{-h}^0 p(x = 0, y, t) dy$$

$$= \frac{2\rho g A \cos \omega t}{\cosh kh} \int_{-h}^0 \cosh k(y+h) dy - \int_{-h}^0 \rho g y dy$$

$$= \frac{2\rho g A}{k} \tanh kh \cos \omega t + \frac{\rho g h^2}{2}$$

MIT OpenCourseWare  
<http://ocw.mit.edu>

2.019 Design of Ocean Systems  
Spring 2011

For information about citing these materials or our Terms of Use, visit: <http://ocw.mit.edu/terms>.